

INTRODUCTION

In this work, we consider the poroelasticity problems in heterogeneous porous media Mathematical model contains coupled system of the equations for pressure and displace ments. For the numerical solution we research and implement a Generalized Multiscale Finite Element Method(GMsFEM). This method solves a problem on a coarse grid by creation of the local multiscale basic functions.

We compare the solutions by choosing different numbers of multiscale basis functions and results show that GMsFEM can provide good accuracy.

MATHEMATICAL MODEL

We consider linear poroelasticity problem where we wish to find a pressure p and displacements u satisfying

$$-\operatorname{div} \sigma(u) + \alpha \operatorname{grad}(p) = f_u(x, t), \ x \in \Omega, \ 0 < t \le T,$$
(1)

$$\frac{\partial \operatorname{div} u}{\partial t} + \frac{1}{M} \frac{\partial p}{\partial t} - \operatorname{div} \left(\frac{k}{v} \operatorname{grad} p\right) = f_p(x, t), \ x \in \Omega, \ 0 < t \le T,$$
(2)

Here the primary sources of the heterogeneities in the physical properties arise from elasticity tensor in stress σ and permeability k. We denote M to be the Biot modulus, ν is the fluid viscosity, σ is the linear stress, ε is the strain tensor and α is the Biot-Willis fluid-solid coupling coefficient. σ and ε are given as

$$\sigma(u) = 2\mu\varepsilon(u) + \lambda \operatorname{div}(u)I,$$
$$\varepsilon(u) = \frac{1}{2}(\operatorname{grad} u + \operatorname{grad} u^T),$$

where μ , λ are Lame coefficients, *I* is the identity tensor. In the case where the media has heterogeneous material properties the coefficients μ and λ may be highly variable. The problem for the system of equations (1) - (2) is considered in a bounded domain Ω . For displacements on the boundary $\partial \Omega = \Gamma_D + \Gamma_N$ we set

$$u = 0, \ x \in \Gamma_D, \ -\sigma n = 0, \ x \in \Gamma_N, \tag{3}$$

Where n is the unit normal to the boundary. Similarly, for pressure, we set Dirichlet and Neuman boundary conditions

$$p = 0, \ x \in \Gamma_3 \cup \Gamma_4 \cup \Gamma_5 \tag{4}$$

At the initial time t, the following condition is given

 $\varepsilon($

$$(x,0) = 0, \ p(x,0) = p_0, \ x \in \Omega,$$
(5)

for deformations and pressure.

Space approximation

lpha ·

For approximation by space, we use finite element method abd write the variational formulation for equations (1) - (2) with the initial boundary conditions (3) - (5): find $u \in W, p \in V$ such that

$$a(u, v) + g(p, v) = l^u(v), v \in W'$$

 $d(\dot{u}, q) + b(p, q) = l^p(q), q \in V'.$

Here the bilinear and linear forms are given as follows:

$$\begin{aligned} a(u,v) &= \int_{\Omega} \sigma(u)\varepsilon(v)dx, \ g(p,v) = \alpha \int_{\Omega} \operatorname{grad} p \ v dx, \\ d(\dot{u},q) &= \alpha \int_{\Omega} \frac{\partial \operatorname{div} u}{dt} q dx, \ c(p,q) = \frac{1}{M} \int_{\Omega} p q dx, \\ b(p,q) &= \int_{\Omega} (k \operatorname{grad} p, \operatorname{grad} q) dx, \\ l^{u}(v) &= \int_{\Omega} f_{u} v dx, \ l^{p}(q) = \int_{\Omega} f_{p} q dx + \int_{\partial \Omega} p_{1} q ds. \end{aligned}$$

Bilinear forms $a(\cdot, \cdot), b(\cdot, \cdot)$ and $c(\cdot, \cdot)$ are symmetric and positively defined, d(v, v) =-g(v, v) for $v \in V, v \in W$. Approximation by time

For approximation by time, we use an implicit difference scheme

$$\int_{\Omega} \sigma^{e}(u^{n+1})\varepsilon(v)dx + \alpha \int_{\Omega} \operatorname{grad} p^{n}vdx = f_{u}(x,t)$$
$$\alpha \int_{\Omega} \frac{\operatorname{div}(u^{n+1} - u^{n})}{\tau} qdx + \frac{1}{M} \int_{\Omega} p^{n+1}qdx + \int_{\Omega} (k \operatorname{grad} p^{n+1}, \operatorname{grad} q)dx = f_{p}(x,t),$$

where we use linear basis functions for pressure and for displacements.

GMSFEM FOR POROELASTICITY

In this section, we describe in detail the Generalized Multiscale Finite Element Method (GMsFEM) for solution of the poroelasticity problems in heterogeneous media. GMsFEM contains three steps: (1) Construction of the coarse and fine meshes and local domains where we construct multiscale basis functions, (2) Solve local spectral problems for construction of the multiscale basis functions and (3) Construction and solution of the coarse scale approximation on multiscale space. **Step 1.** Construction of the coarse and fine meshes (\mathcal{T}_H and \mathcal{T}_h) and local domains (ω_i)

Step 2. Spectral problem. For construction of the multiscale basis functions. we solve a local spectral problems in domain ω_i for displacement and pressure separately. The local spectral problems

Pressure

$$a^{u}(u, v) = \lambda_{u}, s^{u}(u, v),$$

 $a^{u}(u, v) = \int_{\omega_{s}} \sigma(u)\varepsilon(v)dx, s^{u} =$

to the first smallest L eigenvalues, where $\lambda_1 \leq \lambda_2 \leq \ldots \leq \lambda_L$. We can write spectral problems in the matrix form

Press
$$A^u \varphi$$

where A^{u} , S^{u} matrices for displacements and A^{p} , S^{p} for pressure. In the fact that the spectral problem must be solved many times. Therefore, it is possible to reduce the dimension of the problem. The method consists in solving the spectral problem only at the boundary nodes of the domain ω_i . That is, we solve problems (1) in another spaces, this spaces is called a snapshot space, denoted as V_{snap} for displacement and Q_{snap} for pressure. To go into spaces V_{snap} , Q_{snap} , you need to obtain a transition matrices R_{snap}^{u} , R_{snap}^{p} .

Pressure
$$R^u_{snap}$$
 =

Next, we move to spaces V_{snap} , Q_{snap} and obtain the following eigenvalue problem

Pressure $\overline{A}^u \overline{\varphi} = \lambda_i$

RESULTS

In this section, we present numerical examples For the Biot modulus, we take M = 6250000poroelasticity problem the number of basis fund



Heterogeneous permeability a

| Basis | N_c^u | N_c^p | Displacement errors L2 | Displacement error | | | | | |
|--|---------|---------|------------------------|--------------------|--|--|--|--|--|
| 1 | 242 | 121 | 16.4053 | 22.4455 | | | | | |
| 2 | 484 | 242 | 6.76872 | 12.4989 | | | | | |
| 4 | 968 | 484 | 7.67148 | 9.8243 | | | | | |
| 8 | 1936 | 968 | 6.92451 | 5.4054 | | | | | |
| 12 | 2904 | 1936 | 2.06456 | 3.64363 | | | | | |
| Numerical comparision of the solutions b | | | | | | | | | |

Numericar comp basis functions i

GMSFEM FOR POROELASTICITY PROBLEMS IN HETEROGENEOUS MEDIA

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$$= \int_{\omega_i} (\lambda + \mu)(u, v) dx, \quad \begin{vmatrix} \text{Displacements} \\ a^p(p, q) &= \lambda_p s^p(p, q), \\ a^p(p, q) &= \int_{\omega_i} k \operatorname{grad} p, \operatorname{grad} q \, dx, \ s^p &= \int_{\omega_i} (kp, q) dx, \end{vmatrix}$$

where v, q is a trial functions. A multiscale spaces V_h, Q_h , we will form using eigenvectors $\varphi_1, \varphi_2, \dots, \varphi_L, \psi_1, \psi_2, \dots, \psi_L$ corresponding

are

$$= \lambda_u S^u \varphi, \quad x \in \omega_i, \quad \begin{vmatrix} \text{Displacements} \\ A^p \psi = \lambda_p S^p \psi, \quad x \in \omega_i \end{vmatrix}$$

$$= [\varphi_1^{snap}, ..., \varphi_{L_i}^{snap}], \quad \begin{vmatrix} \text{Displacements} \\ R_{snap}^p = [\psi_1^{snap}, ..., \psi_{L_i}^{snap}], \end{vmatrix}$$

$$_{\mu}\overline{S}^{u}\overline{\varphi}, \quad x \in V_{snap}, \quad \begin{vmatrix} \text{Displacements} \\ \overline{A}^{p}\overline{\psi} = \lambda_{p}\overline{S}^{p}\overline{\psi}, \quad x \in Q_{snap}. \end{vmatrix}$$
 As a result, we obtain a solution on a fine grid for

to demonstrate the performance of the CMSFEM for computing the solution of the porcelasticity problem in heterogenous domains.

$$0$$
 and set $\alpha = 1.0$, the Poisson's ratio is $\nu = 0.3$. Permeability field and elastic modulus are shown in figure above. When solving of
heterons for displacement and pressure is the same. Time parameters: $T_{max} = 0.001$ and $dt = 0.0001$.

 0 and set $\alpha = 1.0$, the Poisson's ratio is $\nu = 0.3$. Permeability field and elastic modulus are shown in figure above. When solving of
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will improve.

Pressure

 $\overline{A}^u = R^u_{sna}$

For obtaining conforming basis functions we use linear partition of unity functions. **Step 3.** *Assembling a matrices in a multiscale space* transition matrix

$$R = \begin{pmatrix} R_u \\ R_p \end{pmatrix} = \begin{pmatrix} 0 & \cdots & B_1 \\ 0 & \cdots & \\ \vdots & \vdots & \\ 0 & \cdots & B_L \\ 0 & \cdots & \\ 0 & \cdots & \\ \vdots & \vdots & \\ 0 & \cdots & \\ 0 & \cdots & \\ 0 & \cdots & B_{L_a} \\ 0 & \cdots & \\ 0 & \cdots$$

where subindexes x, y, p denote the displacement in x, y directions and the pressure, respectively. B_j and P_i are multiscale basis functions and linear partition of unity functions, j = 0...L, $i = 0...3 * N_c$, L is the number of bases, N_c is the number of vertices of a coarse grid, N_f id the number of vertices of a fine grid. Thus, the dimension of the matrix R is equal to $3 * N_c L \times 3 * N_f$. Then the system of equations can be translated into a coarse grid

here
$$A_c = R_u A_f R_u^T$$
, $B_c = R_p B_f R_p^T$, $F_c =$
$$A_f u = \int_{\Omega} \sigma(u) \varepsilon(v) dx, \quad B_f p = \frac{1}{N}$$
$$G_f p = \int_{\Omega} \alpha \operatorname{grad}(p) \cdot v \, dx, \quad D_f u$$

after obtaining of a coarse-scale solution, we can reconstruct fine-scale solition



$$\begin{array}{l} \begin{array}{l} \mbox{Pressure} \\ \overline{A}^{u} = R^{u}_{snap} A^{u} (R^{u}_{snap})^{T}, \\ \overline{A}^{u} = R^{u}_{snap} A^{u} (R^{u}_{snap})^{T}, \end{array} \begin{array}{l} \begin{array}{l} \mbox{Displacements} \\ \overline{S}^{u} = R^{u}_{snap} S^{u} (R^{u}_{snap})^{T}, \\ \overline{S}^{p} = R^{p}_{snap} S^{p} (R^{p}_{snap})^{T}. \end{array} \end{array}$$

where $\varphi_j = (R_{snap}^u)^T \overline{\varphi}$ and $\psi_j = (R_{snap}^p)^T \overline{\psi}$. As a result of solving the spectral problem, we have obtained the bases that are used to move from small scale to coarse. Bases can be different numbers, with the increase in the number of bases, the accuracy of the solution

Next, we construct transition matrix R from a fine grid to a coarse grid and use it for reducing the dimension of the problem. The

| $\begin{array}{c} & * P_1 \\ 0 \\ 0 \end{array}$ | · · · · · · · · | $\begin{array}{c} 0\\ B_{1_y} \ast P_1\\ 0\end{array}$ | · · · · · · · | $\begin{array}{c} 0\\ 0\\ B_{1_p}*P_1\end{array}$ | · · · · · · · | 0 0 0 | |
|--|--------------------|--|------------------|--|------------------|------------------|---|
| $\begin{array}{c} \ddots \\ A_x & * P_1 \\ 0 \\ 0 \end{array}$ | | $\begin{array}{c} \ddots \\ 0 \\ B_{Ly} * P_1 \\ 0 \end{array}$ | | $\begin{array}{c} \ddots \\ 0 \\ 0 \\ B_{L_p} * P_1 \end{array}$ | | : 0 0 0 | |
| $\begin{array}{c} \ddots \\ {}_{x} \ast P_{N_{c}} \\ 0 \\ 0 \end{array}$ | | $\begin{array}{c} \ddots \\ 0 \\ B_{Ly} * P_{Nc} \\ 0 \end{array}$ | | $\begin{array}{c} \ddots \\ 0 \\ 0 \\ B_{L_p} * P_{N_c} \end{array}$ | | : 0 0 0 |) |

$$\begin{aligned} A_c u_c + G_c p_c &= F_c, \\ D_c u_c + B_c p_c &= Y_c, \end{aligned}$$
$$R_u F_f, Y_c = R_p Y_f, G_c = R_u G_f R_p^T, D_c = R_p D_f R_u^T \text{ and} \end{aligned}$$
$$\frac{1}{4} \int_{\Omega} \frac{p}{\tau} q dx + \int_{\Omega} (k \operatorname{grad} p, \operatorname{grad} q) dx, \end{aligned}$$

$$u = \alpha \int_{\Omega} \frac{\operatorname{div} u}{\tau} q dx, \quad F_f = \int_{\Omega} f_u v dx, \quad Y_f = \int_{\Omega} f_p q dx + \int_{\partial \Omega} p_1 q dx$$

$$u_{ms} = R_u^T u_c, \quad p_{ms} = R_p^T p_c$$

or the problem of poroelasticity.

ONCLUSION

this work, a Generalized Multiscale Finite Element Methods for the poroelasticity oblem in heterogeneous porous media is applied. For the numerical solution of the sing coupled system of equations for pressure and displacements, an approximation the problem is constructed usingfinite element method with linear basis functions. e show the errors relative with varying multiscale basis functions and distribution of placement along X and Y direction and pressure at the final time.

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